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Steady fluid-structure interaction using fictitious domain

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Abstract. We present a formulation for a steady fluid-structure interaction problem using fictitious domain technique with penalization. Numerical results are presented.

Keywords: fluid-structure interaction, fictitious domain

1 Setting for a steady fluid-structure interaction problem

Let $D \subset \mathbb{R}^2$ be a bounded open domain with boundary ∂D . Let Ω_0^S be the undeformed structure domain, and suppose that its boundary admits the decomposition $\partial\Omega_0^S = \Gamma_D \cup \Gamma_0$, where Γ_0 is a relatively open subset of the boundary.

On Γ_D we impose zero displacement for the structure. We assume that $\overline{\Omega_0^S} \subset D$.

Suppose that the structure is elastic and denote by $\mathbf{u} = (u_1, u_2) : \Omega_0^S \rightarrow \mathbb{R}^2$ its displacement. A particle of the structure with initial position at the point \mathbf{X} will occupy the position $\mathbf{x} = \varphi(\mathbf{X}) = \mathbf{X} + \mathbf{u}(\mathbf{X})$ in the deformed domain $\Omega_u^S = \varphi(\Omega_0^S)$.

We assume that $\overline{\Omega_u^S} \subset D$ and the fluid occupies $\Omega_u^F = D \setminus \overline{\Omega_u^S}$. We set $\Gamma_u = \varphi(\Gamma_0)$, then the boundary of the deformed structure is $\partial\Omega_u^S = \Gamma_D \cup \Gamma_u$ and the boundary of the fluid domain admits the decomposition $\partial\Omega_u^F = \partial D \cup \Gamma_D \cup \Gamma_u$. The fluid-structure geometrical configuration is represented in Figure 1.

Generally, the fluid equations are described using Eulerian coordinates, while for the structure equations, the Lagrangian coordinates are employed. The gradients with respect to the Eulerian coordinates $\mathbf{x} \in \Omega_u^S$ of a scalar field $q : D \rightarrow \mathbb{R}$ or a vector field $\mathbf{w} = (w_1, w_2) : D \rightarrow \mathbb{R}^2$ are denoted by ∇q and $\nabla \mathbf{w}$. The divergence operators with respect to the Eulerian coordinates of a vector field $\mathbf{w} = (w_1, w_2) : D \rightarrow \mathbb{R}^2$ and of a tensor $\sigma = (\sigma_{ij})_{1 \leq i, j \leq 2}$ are denoted by $\nabla \cdot \mathbf{w}$ and $\nabla \cdot \sigma$.

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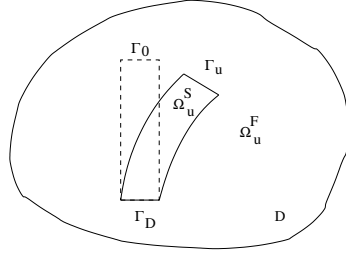


Fig. 1. Geometrical configuration.

When the derivatives are with respect to the Lagrangian coordinates $\mathbf{X} = \varphi^{-1}(\mathbf{x}) \in \Omega_0^S$, we use the notations: $\nabla_{\mathbf{X}} \mathbf{u}$, $\nabla_{\mathbf{X}} \cdot \mathbf{u}$, $\nabla_{\mathbf{X}} \cdot \sigma$.

If \mathbf{A} is a square matrix, we denote by $\det \mathbf{A}$, \mathbf{A}^{-1} , \mathbf{A}^T its determinant, the inverse and the transposed matrix, respectively. We write $\text{cof } \mathbf{A} = (\det \mathbf{A}) (\mathbf{A}^{-1})^T$ the co-factor matrix of \mathbf{A} . We write $\mathbf{A}^{-T} = (\mathbf{A}^{-1})^T$.

We denote by $\mathbf{F}(\mathbf{X}) = \mathbf{I} + \nabla_{\mathbf{X}} \mathbf{u}(\mathbf{X})$ the gradient of the deformation and by $J(\mathbf{X}) = \det \mathbf{F}(\mathbf{X})$ the Jacobian determinant, where \mathbf{I} is the unit matrix.

We introduce the tensor $\epsilon(\mathbf{w}) = \frac{1}{2} (\nabla \mathbf{w} + (\nabla \mathbf{w})^T)$ and we assume that the fluid is Newtonian and the Cauchy stress tensor is given by $\sigma^F(\mathbf{v}, p) = -p \mathbf{I} + 2\mu^F \epsilon(\mathbf{v})$, where $\mu^F > 0$ is the viscosity of the fluid and \mathbf{I} is the unit matrix. We assume that the structure verifies the linear elasticity equation, under the assumption of small deformations. The stress tensor of the structure written in the Lagrangian framework is $\sigma^S(\mathbf{u}) = \lambda^S (\nabla \cdot \mathbf{u}) \mathbf{I} + 2\mu^S \epsilon(\mathbf{u})$, where $\lambda^S, \mu^S > 0$ are the Lamé coefficients.

The problem is to find the structure displacement $\mathbf{u} : \overline{\Omega}_0^S \rightarrow \mathbb{R}^2$, the fluid velocity $\mathbf{v} : \overline{\Omega}_u^F \rightarrow \mathbb{R}^2$ and the fluid pressure $p : \overline{\Omega}_u^F \rightarrow \mathbb{R}$ such that:

$$-\nabla_{\mathbf{X}} \cdot \sigma^S(\mathbf{u}) = \mathbf{f}^S, \quad \text{in } \Omega_0^S \quad (1)$$

$$\mathbf{u} = 0, \quad \text{on } \Gamma_D \quad (2)$$

$$-\nabla \cdot \sigma^F(\mathbf{v}, p) = \mathbf{f}^F, \quad \text{in } \Omega_u^F \quad (3)$$

$$\nabla \cdot \mathbf{v} = 0, \quad \text{in } \Omega_u^F \quad (4)$$

$$\mathbf{v} = 0, \quad \text{on } \partial D \quad (5)$$

$$\mathbf{v} = 0, \quad \text{on } \Gamma_D \quad (6)$$

$$\mathbf{v} = 0, \quad \text{on } \Gamma_u \quad (7)$$

$$\omega(\sigma^F(\mathbf{v}, p) \mathbf{n}^F) \circ \varphi = -\sigma^S(\mathbf{u}) \mathbf{n}^S, \quad \text{on } \Gamma_0 \quad (8)$$

where $\mathbf{f}^S : \Omega_0^S \rightarrow \mathbb{R}^2$ are the applied volume forces on the structure and \mathbf{n}^S is the structure unit outward vector normal to $\partial\Omega_0^S$. Similarly, we define $\mathbf{f}^F : \Omega_u^F \rightarrow \mathbb{R}^2$ and \mathbf{n}^F the fluid unit outward vector normal to $\partial\Omega_u^F$.

We point out that the stress tensor of the structure is defined on the undeformed structure domain Ω_0^S , while the Cauchy stress tensors of the fluid is defined in the deformed domain Ω_u^F .

We have used the notation $\omega(\mathbf{X}) = \|J \mathbf{F}^{-T} \mathbf{n}^S\|_{\mathbb{R}^2} = \|\text{cof}(\mathbf{F}) \mathbf{n}^S\|_{\mathbb{R}^2}$ for \mathbf{X} on $\partial\Omega_0^S$, which is a kind of Jacobian determinant for the change of variable formula for integral over surface.

The equations (1), (2) concern the structure, while (3)-(6) concern the fluid. The equations (7), (8) represent the boundary conditions on the moving fluid-structure interface. The fluid and the structure domains Ω_u^F , Ω_u^S depend on the structure displacement u which is unknown.

2 Parametrization and regularization of the characteristic function

The regularization of the characteristic function of the deformed structure domain is necessary in order to prove the continuity of the solution with respect to the structure displacement, [3].

Denote by $\|\cdot\|_{1,\infty,\Omega}$ the usual norm of the Sobolev space $W^{1,\infty}(\Omega)$ and by $\|\cdot\|_{m,\Omega}$ the usual norm of $H^m(\Omega)$, $m \geq 0$ with the convention $H^0(\Omega) = L^2(\Omega)$.

For every $0 < \delta < 1$, there exists $0 < \eta_\delta < 1$ such that

$$1 - \delta \leq \det(\mathbf{I} + \nabla \mathbf{u}) \leq 1 + \delta, \quad \text{a.e. } \mathbf{x} \in \Omega_0^S \quad (9)$$

for all $\mathbf{u} \in (W^{1,\infty}(\Omega_0^S))^2$ that satisfy $\|\mathbf{u}\|_{1,\infty,\Omega_0^S} \leq \eta_\delta$.

We define

$$B_\delta = \{u \in W^{1,\infty}(\Omega_0^S)^2; \|u\|_{1,\infty,\Omega_0^S} \leq \eta_\delta, u = 0 \text{ on } \Gamma_D\}. \quad (10)$$

Let $j \in W^{1,\infty}(D)$ be a parametrization of $\Omega_0^S \subset D$, i.e. :

$$j(\mathbf{x}) > 0, \quad \mathbf{x} \in \Omega_0^S, \quad j(\mathbf{x}) < 0, \quad \mathbf{x} \in D \setminus \overline{\Omega_0^S}, \quad j(\mathbf{x}) = 0, \quad \mathbf{x} \in \partial\Omega_0^S.$$

The parametrization is not necessarily unique.

Let $\mathbf{u} \in B_\delta$ be a given structure displacement. Denote, as before, $\Omega_u^S = \varphi(\Omega_0^S)$, where $\varphi(\mathbf{X}) = \mathbf{X} + \mathbf{u}(\mathbf{X})$. Then $\varphi : \overline{\Omega_0^S} \rightarrow \overline{\Omega_u^S}$ is bijective and bilipschitzian and

$$j_u(\mathbf{y}) = \begin{cases} j(\mathbf{x}), & \mathbf{y} = \varphi(\mathbf{x}) \in \Omega_u^S \\ 0, & \mathbf{y} \in \partial\Omega_u^S \\ -\text{dist}(\mathbf{y}, \overline{\Omega_u^S}), & \mathbf{y} \notin \overline{\Omega_u^S} \end{cases}$$

is a parametrization of Ω_u^S , $j_u \in W^{1,\infty}(D)$.

If H is the Heaviside function $H : \mathbb{R} \rightarrow \{0, 1\}$,

$$H(r) = \begin{cases} 1, & r \geq 0 \\ 0, & r < 0 \end{cases}$$

then $H(j_u(\cdot))$ is the characteristic function of Ω_u^S .

For $\varepsilon > 0$, let $\Omega_0^\varepsilon \subset\subset \Omega_0^S$. Since $j : D \rightarrow \mathbb{R}$ is Lipschitz continuous and $j > 0$ in Ω_0^S , there is $\mu_\varepsilon > 0$ such that $j(\mathbf{x}) \geq \mu_\varepsilon > 0$, for all $\mathbf{x} \in \Omega_0^\varepsilon$. Consequently,

$$\mu_\varepsilon \leq \min_{\mathbf{y} \in \overline{\Omega_u^\varepsilon}} j_u(y), \quad \forall \mathbf{u} \in B_\delta.$$

Then we take H^{μ_ε} , the Yosida regularization of H

$$H^{\mu_\varepsilon}(r) = \begin{cases} 1, & r \geq \mu_\varepsilon \\ \frac{r}{\mu_\varepsilon} & 0 \leq r < \mu_\varepsilon \\ 0, & r < 0 \end{cases}$$

and we set $\tilde{H}_u(\mathbf{x}) = H^{\mu_\varepsilon}(j_u(\mathbf{x}))$ for all $\mathbf{x} \in D$, which is a Lipschitz regularization of the characteristic function of Ω_u^S . We have constructed $\tilde{H}_u : \overline{D} \rightarrow \mathbb{R}$, Lipschitz on \overline{D} , $0 \leq \tilde{H}_u(\mathbf{x}) \leq 1$, for all \mathbf{x} in \overline{D} such that

$$\tilde{H}_u(\mathbf{x}) = \begin{cases} 0, & \mathbf{x} \in \overline{D} \setminus \Omega_u^S \\ 1, & \mathbf{x} \in \Omega_u^\varepsilon \end{cases} \quad (11)$$

where $\Omega_u^\varepsilon \subset\subset \Omega_u^S$.

3 Weak formulation using fictitious domain technique with penalization

We assume that D and Ω_0^S are Lipschitz. Let us introduce the bi-linear forms

$$a_S(\mathbf{u}, \mathbf{w}^S) = \int_{\Omega_0^S} (\lambda^S (\nabla \cdot \mathbf{u}) (\nabla \cdot \mathbf{w}^S) + 2\mu^S \epsilon(\mathbf{u}) : \epsilon(\mathbf{w}^S)) d\mathbf{X}$$

$$a_F(\mathbf{v}, \mathbf{w}) = \int_D 2\mu^F \epsilon(\mathbf{v}) : \epsilon(\mathbf{w}) d\mathbf{x}$$

$$b_F(\mathbf{w}, p) = - \int_D (\nabla \cdot \mathbf{w}) p d\mathbf{x}$$

and the Hilbert spaces

$$W^S = \left\{ \mathbf{w}^S \in (H^1(\Omega_0^S))^2; \mathbf{w}^S = 0 \text{ on } \Gamma_D \right\},$$

$$W = (H_0^1(D))^2,$$

$$Q = L_0^2(D) = \left\{ q \in L^2(D); \int_D q d\mathbf{x} = 0 \right\}.$$

We assume that $\mathbf{f}^F \in (L^2(D))^2$, $\mathbf{f}^S \in (L^2(\Omega_0^S))^2$.

Weak fluid formulation using fictitious domain

For a given $\mathbf{u} \in B_\delta$, we define: fluid velocity $\mathbf{v}_\varepsilon \in W$ and fluid pressure $p_\varepsilon \in Q$, as the solution of the following problem:

$$a_F(\mathbf{v}_\varepsilon, \mathbf{w}) + b_F(\mathbf{w}, p_\varepsilon) + \frac{1}{\varepsilon} \int_D \tilde{H}_u(\mathbf{v}_\varepsilon \cdot \mathbf{w} + \nabla \mathbf{v}_\varepsilon : \nabla \mathbf{w}) d\mathbf{X} = \int_D \mathbf{f}^F \cdot \mathbf{w} d\mathbf{X}, \forall \mathbf{w} \in W \quad (12)$$

$$b_F(\mathbf{v}_\varepsilon, q) = 0, \forall q \in Q \quad (13)$$

Weak structure formulation

For given $\mathbf{u} \in B_\delta$, $\mathbf{v}_\varepsilon \in W$ and $p_\varepsilon \in Q$, we define the structure displacement $\mathbf{u}_\varepsilon \in W^S$ as the solution of

$$\begin{aligned} a_S(\mathbf{u}_\varepsilon, \mathbf{w}^S) &= \int_{\Omega_0^S} \mathbf{f}^S \cdot \mathbf{w}^S d\mathbf{X} + \int_{\Omega_0^S} J(\sigma^F(\mathbf{v}_\varepsilon, p_\varepsilon) \circ \varphi) \mathbf{F}^{-T} : \nabla_{\mathbf{X}} \mathbf{w}^S d\mathbf{X} \\ &+ \frac{1}{\varepsilon} \int_{\Omega_0^S} J \tilde{H}_u(\varphi) ((\mathbf{v}_\varepsilon \circ \varphi) \cdot \mathbf{w}^S + (\nabla \mathbf{v}_\varepsilon \circ \varphi) \mathbf{F}^{-T} : \nabla_{\mathbf{X}} \mathbf{w}^S) d\mathbf{X} \\ &- \int_{\Omega_0^S} J(\mathbf{f}^F \circ \varphi) \cdot \mathbf{w}^S d\mathbf{X}, \forall \mathbf{w}^S \in W^S \end{aligned} \quad (14)$$

where $\varphi(\mathbf{X}) = \mathbf{X} + \mathbf{u}(\mathbf{X})$, $\mathbf{F}(\mathbf{X}) = \mathbf{I} + \nabla_{\mathbf{X}} \mathbf{u}(\mathbf{X})$, $J(\mathbf{X}) = \det \mathbf{F}(\mathbf{X})$.

Remark 1. From the structure equation $-\nabla \cdot \sigma^S(\mathbf{u}_\varepsilon) = \mathbf{f}^S$, in Ω_0^S using Green's formula, we obtain for all $\mathbf{w}^S = 0$ on Γ_D that

$$a_S(\mathbf{u}_\varepsilon, \mathbf{w}^S) = \int_{\Omega_0^S} \mathbf{f}^S \cdot \mathbf{w}^S d\mathbf{X} + \int_{\Gamma_0} \sigma^S(\mathbf{u}_\varepsilon) \mathbf{n}^S \cdot \mathbf{w}^S dS.$$

We can prove (see [3]) that the sum of the last three terms in (14) is equal to the fluid forces acting on the structure which is also equals to $\int_{\Gamma_0} \sigma^S(\mathbf{u}_\varepsilon) \mathbf{n}^S \cdot \mathbf{w}^S dS$. In fact, from (14) and the above weak formulation of the structure, we can get that the boundary condition at the interface concerning the continuity of the stress (8) is verified in a weak sense (see [3]). The second boundary condition at the interface is the continuity of the velocity (7). This is obtained by using the penalization term in the structure domain in (12).

For each $i \in \mathbb{N}^*$, there exists an unique eigenvalue $\lambda_i > 0$ and an unique eigenfunction $\phi^i \in \mathbf{W}^S$, solution of

$$a_S(\phi^i, \mathbf{w}^S) = \lambda_i \int_{\Omega_0^S} \phi^i \cdot \mathbf{w}^S d\mathbf{X}, \quad \forall \mathbf{w}^S \in \mathbf{W}^S \quad (15)$$

such that

$$\int_{\Omega_0^S} \phi^i \cdot \phi^j d\mathbf{X} = \delta_{ij}, \quad (16)$$

see [8], chap. 6. We assume that $\lambda_1 \leq \lambda_2 \leq \dots$. The set $\{\phi^i, i \in \mathbb{N}^*\}$ forms an orthonormal basis of $L^2(\Omega_0^S)$. Let $m \in \mathbb{N}^*$ be given. Let \mathbf{u}_ε^m be the orthogonal projection of \mathbf{u}_ε on $\text{span}(\phi^i, i = 1, \dots, m)$ in $L^2(\Omega_0^S)$, so $\mathbf{u}_\varepsilon^m = \sum_{i=1}^m \alpha_i \phi^i$, $\alpha_i \in \mathbb{R}$.

We define

$$B_\delta^m = \left\{ \mathbf{u} \in (W^{1,\infty}(\Omega_0^S))^2; \mathbf{u} = 0 \text{ on } \Gamma_D, \|\mathbf{u}\|_{1,\infty,\Omega_0^S} < \eta_\delta, \right. \\ \left. \mathbf{u} = \sum_{i=1}^m \alpha_i \phi^i, \alpha_i \in \mathbb{R} \right\}. \quad (17)$$

We have $B_\varepsilon^m \subset B_\delta$. For each $\mathbf{u} \in B_\delta$, we define the nonlinear operator

$$T_\varepsilon^m(\mathbf{u}) = \mathbf{u}_\varepsilon^m.$$

A solution of the penalized fluid-structure interaction problem will be, by definition, a fixed point of T_ε^m in B_ε^m .

Remark 2. As in [3], we can prove the existence of a solution of the penalized fluid-structure interaction problem $T_\varepsilon^m(\mathbf{u}_\varepsilon^m) = \mathbf{u}_\varepsilon^m$ using the Schauder fixed point theorem. In order to obtain supplementary regularity of the Stokes equations, a non linear penalization term $\frac{1}{\varepsilon} \int_D \tilde{H}_u \text{sgn}(\mathbf{v}_\varepsilon) |\mathbf{v}_\varepsilon|^{\alpha-1} \cdot \mathbf{w} \, d\mathbf{x}$ was employed in [3], where $\alpha > 2$. In the present paper, we use a linear penalization term in (12), but we are working only with a finite number of eigenfunctions of the structure equations. The behavior of \mathbf{u}_ε^m when ε goes to zero will be studied in [4].

4 Fixed point iterations

We replace \tilde{H}_u in (12) and (14) by χ_u^S the characteristic function of Ω_u^S in order to simplify the computation. The regularization of the the characteristic function was necessary to obtain continuity of the solution with respect to the structure displacement.

Under the assumption of small displacements for the structure, we can approach the Jacobian determinant J by 1 and the gradient of the deformation \mathbf{F} by the unit matrix \mathbf{I} .

Algorithm 1 by fixed point iterations

Step 1. Given the initial displacement of the structure $\mathbf{u}^{m,0} = \sum_{i=1}^m \alpha_i^0 \phi^i$, compute the characteristic function $\chi_{u^0}^S$, put $k := 0$.

Step 2. Find the velocity $\mathbf{v}_\varepsilon^k \in (H^1(D))^2$, $\mathbf{v}_\varepsilon^k = \mathbf{g}$ on ∂D and the pressure $p_\varepsilon^k \in Q$ by solving the fluid problem

$$a_F(\mathbf{v}_\varepsilon^k, \mathbf{w}) + b_F(\mathbf{w}, p_\varepsilon^k) \\ + \frac{1}{\varepsilon} \int_D \chi_{u^k}^S (\mathbf{v}_\varepsilon^k \cdot \mathbf{w} + \nabla \mathbf{v}_\varepsilon^k : \nabla \mathbf{w}) \, d\mathbf{x} = \int_D \mathbf{f}^F \cdot \mathbf{w} \, d\mathbf{x}, \quad \forall \mathbf{w} \in W \\ b_F(\mathbf{v}_\varepsilon^k, q) = 0, \quad \forall q \in Q.$$

Step 3. Find the new displacement of the structure $\mathbf{u}_\varepsilon^{m,k+1} = \sum_{i=1}^m \alpha_i^{k+1} \phi^i$ by solving

$$\begin{aligned} \alpha_i^{k+1} \lambda_i &= \int_{\Omega_0^S} (\mathbf{f}^S - \mathbf{f}^F) \cdot \phi^i \, d\mathbf{x} \\ &+ \int_{\Omega_0^S} 2\mu^F \boldsymbol{\epsilon}(\mathbf{v}_\varepsilon^k) : \boldsymbol{\epsilon}(\phi^i) \, d\mathbf{x} - \int_{\Omega_0^S} (\nabla \cdot \phi^i) p_\varepsilon^k \, d\mathbf{x} \\ &+ \frac{1}{\varepsilon} \int_{\Omega_0^S} ((\mathbf{v}_\varepsilon^k \circ \varphi_\varepsilon^k) \cdot \phi^i + (\nabla \mathbf{v}_\varepsilon^k \circ \varphi_\varepsilon^k) : \nabla \phi^i) \, d\mathbf{x}, \quad i = 1, \dots, m \end{aligned}$$

where $\varphi_\varepsilon^k(\mathbf{X}) = \mathbf{X} + \mathbf{u}_\varepsilon^{m,k}(\mathbf{X})$.

Step 4. Stopping test: if $\|\mathbf{u}_\varepsilon^{m,k} - \mathbf{u}_\varepsilon^{m,k+1}\|_{0,\Omega_0^S} \leq \text{tol}$, then **Stop**.

Step 5. Compute the characteristic function $\chi_{u_\varepsilon^{m,k+1}}^S$, put $k := k + 1$ and **Go to Step 2**.

5 Least squares approach

The previous algorithm converges if the operator T_ε^m is a contraction. But, the **Algorithm 1** fails for some physical parameters. For this reason, we introduce a second algorithm which is more robust.

For $\boldsymbol{\alpha} = (\alpha_1, \dots, \alpha_m) \in \mathbb{R}^m$, we can define $\boldsymbol{\beta} = (\beta_1, \dots, \beta_m) \in \mathbb{R}^m$ by

$$T_\varepsilon^m \left(\sum_{i=1}^m \alpha_i \phi^i \right) = \sum_{i=1}^m \beta_i \phi^i.$$

We set the cost function $J(\boldsymbol{\alpha}) = \frac{1}{2} \sum_{i=1}^m (\alpha_i - \beta_i)^2$ and now the problem to be solved is $\inf_{\boldsymbol{\alpha} \in \mathbb{R}^m} J(\boldsymbol{\alpha})$. In order to solve the optimization problem, we employ the quasi-Newton iterative method called Broyden, Fletcher, Goldforb, Shano (BFGS) scheme (see for example [2], chap. 9).

Algorithm 2 by the BFGS method

Step 0. Choose a starting point $\boldsymbol{\alpha}^0 \in \mathbb{R}^m$, an $m \times m$ symmetric positive matrix H_0 . Set $k = 0$.

Step 1. Compute $\nabla J(\boldsymbol{\alpha}^k)$.

Step 2. If $\|\nabla J(\boldsymbol{\alpha}^k)\| < \text{tol}$ **Stop**.

Step 3. Set $\mathbf{d}^k = -H_k \nabla J(\boldsymbol{\alpha}^k)$.

Step 4. Determine $\boldsymbol{\alpha}^{k+1} = \boldsymbol{\alpha}^k + \theta_k \mathbf{d}^k$, $\theta_k > 0$ by means of an approximate minimization

$$J(\boldsymbol{\alpha}^{k+1}) \approx \min_{\theta \geq 0} J(\boldsymbol{\alpha}^k + \theta \mathbf{d}^k).$$

Step 5. Compute $\delta_k = \boldsymbol{\alpha}^{k+1} - \boldsymbol{\alpha}^k$.

Step 6. Compute $\nabla J(\boldsymbol{\alpha}^{k+1})$ and $\gamma_k = \nabla J(\boldsymbol{\alpha}^{k+1}) - \nabla J(\boldsymbol{\alpha}^k)$.

Step 7. Compute

$$H_{k+1} = H_k + \left(1 + \frac{\gamma_k^T H_k \gamma_k}{\delta_k^T \gamma_k} \right) \frac{\delta_k \delta_k^T}{\delta_k^T \gamma_k} - \frac{\delta_k \gamma_k^T H_k + H_k \gamma_k \delta_k^T}{\delta_k^T \gamma_k}$$

Step 8. Update $k = k + 1$ and go to the **Step 2**.

The matrices H_k approach the inverse of the Hessian of J . For the inaccurate line search at the **Step 4**, the methods of Goldstein and Armijo were used. If we denote by $g : [0, \infty) \rightarrow \mathbb{R}$ the function $g(\theta) = J(\boldsymbol{\alpha}^k + \theta \mathbf{d}^k)$, we determine $\theta_k > 0$ such that: $g(0) + (1 - \lambda) \theta_k g'(0) \leq g(\theta_k) \leq g(0) + \lambda \theta_k g'(0)$, where $\lambda \in (0, 1/2)$.

In this paper, we compute $\nabla J(\boldsymbol{\alpha})$ by the Finite Differences Method $\frac{\partial J}{\partial \alpha_k}(\boldsymbol{\alpha}) \approx (J(\boldsymbol{\alpha} + \Delta \alpha_k \mathbf{e}_k) - J(\boldsymbol{\alpha})) / \Delta \alpha_k$, where \mathbf{e}_k is the k -th vector of the canonical base of \mathbb{R}^m and $\Delta \alpha_k > 0$ is the grid spacing.

Concerning the convergence rate, the fixed point algorithm is slower than the BFGS Method. If the starting point is not sufficiently close to the solution, the fixed point algorithm diverges. On the contrary, the BFGS Method is less sensitive to the choice of the starting point and, in general, it is convergent to a local minimizer from almost any starting point. This is the main advantage, (see [6]).

6 Numerical results. Deformation of a tall building under the action of wind

We have performed numerical simulations using a 2D model adapted from [1] (see Figure 2).

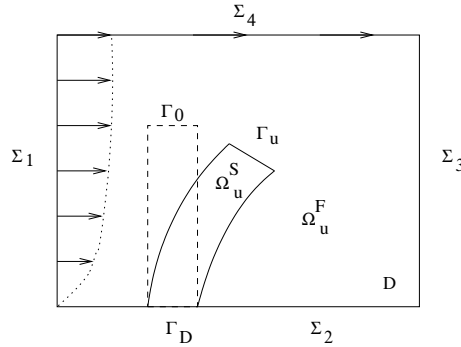


Fig. 2. Geometrical configuration for the numerical results

The dimensions of a rectangular tall building are: height $H = 180 \text{ m}$, length $L = 30 \text{ m}$. The computational domain of the fluid D is a rectangle of height

$H_1 = 3H$ and length $L_1 = L + 4H$, its left bottom corner is at $(0, 0)$. We shall allow nonhomogeneous Dirichlet data in the numerical experiments.

The distance between the left side of the fluid and the left side of the structure is H . We denote by Σ_1, Σ_3 the left and the right vertical boundaries and by Σ_2, Σ_4 the bottom and the top boundaries, respectively.

The mechanical proprieties of the building assumed to be an elastic structure are: Young modulus $E^S = 2.3 \times 10^5 \text{ N/m}^2$, Poisson's ratio $\nu^S = 0.25$, the applied volume forces on the structure $\mathbf{f}^S : \Omega_0^S \rightarrow \mathbb{R}^2$, $\mathbf{f}^S = (0, 0) \text{ N/m}^3$. If the Young modulus is $E^S = 2.3 \times 10^8 \text{ N/m}^2$ as in [1], the displacements of the structure are very small.

The fluid is the air with: dynamic viscosity $\mu^F = 7.03 \times 10^{-2} \text{ N} \cdot \text{s/m}^2$, the applied volume forces on the fluid $\mathbf{f}^F : D \rightarrow \mathbb{R}^2$, $\mathbf{f}^F = (0, 0) \text{ N/m}^3$. The inflow velocity profile is $\mathbf{g}(x_1, x_2) = 100 \left(\frac{x_2}{H}\right)^{0.19} \text{ m/s}$. The considered boundary conditions for the fluid are more natural from the point of view of applications and differ slightly compared with the previous sections. We impose: $\mathbf{v}_\epsilon = \mathbf{g}$ on $\Sigma_1 \cup \Sigma_4$, $\mathbf{v}_\epsilon = 0$ on Σ_2 and $\sigma^F(\mathbf{v}, p) \mathbf{n}^F = 0$ on Σ_3 .

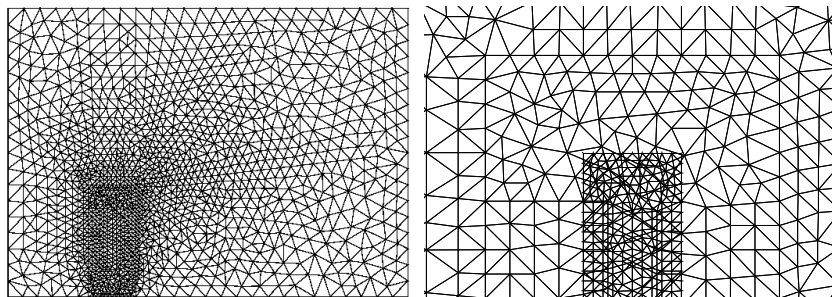


Fig. 3. The fixed mesh of the fluid domain (left). The finer zone $[H - L, H + 2L] \times [0, H + L]$ of the fluid mesh covers the structure mesh which occupies initially the rectangle $[H, H + L] \times [0, H]$ (zoom, right). The fluid and structure meshes are not compatible, for example, a vertex on the structure boundary is not necessary a vertex on the fluid mesh (right).

The numerical tests have been produced using the software *FreeFem++* [5]. For the approximation of the fluid velocity and pressure we have employed the triangular finite elements $\mathbb{P}_1 + \text{bubble}$ and \mathbb{P}_1 respectively on a mesh of 34871 triangles and 17550 vertices. The finite element \mathbb{P}_1 was used in order to solve the structure problem on a mesh of 192 triangles and 125 vertices. The characteristic function was approached by \mathbb{P}_0 finite element.

We have performed the simulation using the **Algorithm 2** described in the previous section. We have used the initial displacement $\alpha^0 = 0$ at the **Step 0** and the tolerance $tol = 0.0001$ for the stopping criterion at the **Step 2**. The penalization parameter is $\varepsilon = 0.001$ and the number of the eigenfunctions is

$m = 5$. The stopping criterion holds after 6 iterations of the BFGS algorithm, the initial value of the cost function is 34.30 and the final value is 5.9×10^{-13} . The maximal structural displacement is 0.148 m .

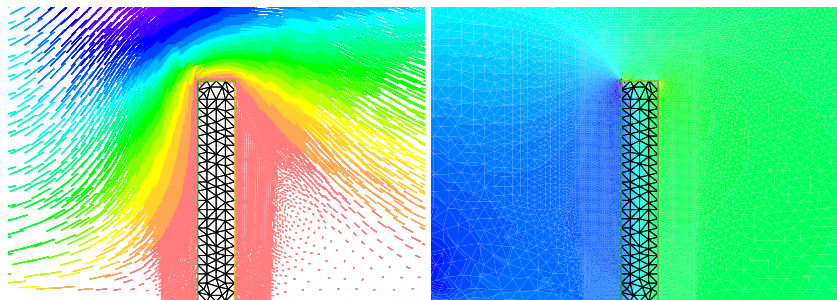


Fig. 4. Velocity (left) and pressure (right) of the fluid around the deformed structure.

The fluid velocity is almost zero in the deformed structure domain, more precisely $\|\mathbf{v}_\varepsilon\|_{1,\Omega_{u_\varepsilon}^S} = \sqrt{\int_D \chi_{u_\varepsilon}^S (\mathbf{v}_\varepsilon \cdot \mathbf{v}_\varepsilon + \nabla \mathbf{v}_\varepsilon : \nabla \mathbf{v}_\varepsilon) dx} = 0.00555$.

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